

# Hydraulics

3rd Year civil

First Term (2009 - 2010)

Chapter ( )

2009 - 2010

3<sup>rd</sup> Year Civil Engineering Open Channel Hydraulics Sheet no. 2 (Uniform flow in open channel)

#### B. Home work

- 1- A channel has two sides vertical and semi-circular bottom of 2ms diameter. Calculate the discharge of water through the channel, when the depth of flow is 2ms. Take C=70 and slope of bed as 1 in 1000.
- 2- A sewer running half full is to be laid at a slope of 1/1000 to serve 200,000 persons at the rate of 300lit/person /day, considering n=0.016, find the sewer diameter if the maximum rate of flow according to which the sewer should be designed can be found by assuming that the total daily discharge flows uniformly in the sewer during 6 hours (not 24 hours).
- A trapezoidal canal of side slope 1:1 and a bed width four times the depth, conveys 40m³/sec, is to be substituted by a semi-circular canal to convey the same discharge at the same velocity. Compare the bed slopes if n=0.012 in both cases.
- 4- Derive the conditions of the best hydraulic section for the triangular and circular sections.
- 5- Determine the dimensions of the most economical trapezoidal channel, n=0.016, to carry a discharge of 8000c.f.s with a slope of 12cm/km.
- 6- A canal having one side vertical and other side is sloping 3:2 caries a discharge of 20m³/sec, with a velocity of 0.5m/sec. determine the canal dimensions and its bed slope such that the section is hydraulically best (n=0.025).
- 7- Show that the maximum discharge in a circular open channel of a certain diameter takes place when the water depth is 0.95 times the channel diameter.
- 8- A special sewer consists of a semi-circular top and bottom of radius (r) joined by parallel vertical sides of length (2r) so that the total height is (4r), it is required to a) determine the angle subtended by water surface at the center of curvature of the upper semicircle to have maximum discharge, b) if the upper surface is raised until it reached the top of the sewer, find the percentage decrease in the flow.

يقطع هذا الكعب ويسلم مع حل التمرين للمعيد أو المدرس المساعد القائم بتدريس السكتان في الوقت الذي يحدد له اسم الطالب:

بسم الله لرحن الرجيم

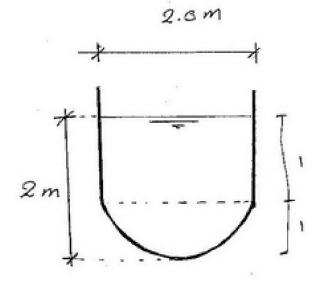
#### Home Work

Reg..

### Criven:

C = 70

S= 1/1000



#### 501.:

$$A = (2 \times 1) + \frac{\pi \times (1)^2}{2} = 3.57 m^2$$

#### 501.:

$$2.78 = \frac{1}{0.016} \times \frac{(0.39 \, d^2)^{5/3}}{(1.57 \, d)^{2/3}} \times (1/1000)^{1/2}$$

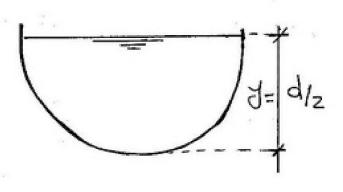
n = 0.012

y b

QTrap = Qcircle

Y-Trap = Vcircle

Req.: Compair bed Slopes



#### Sol . :

$$\frac{S_{T}^{1/2}}{P_{T}^{2/3}} = \frac{S_{c}^{1/2}}{P_{c}^{2/3}}$$

$$\frac{P_{T}}{P_{T}^{2/3}} = \frac{P_{c}^{2/3}}{P_{c}^{2/3}}$$

$$\frac{P_{T}}{P_{C}^{2/3}} = \frac{P_{c}^{2/3}}{P_{c}^{2/3}} = \frac{P_{c}^{2/3}}{P$$

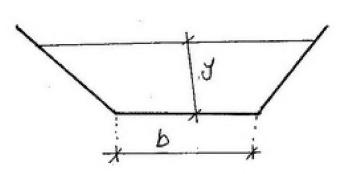
#### J(5):

#### Given:

n = 0.016

Q = 8000 ft3/sec.

S = 12 cm/km



Reg.: Design B. H.S

#### 50l .:

$$\therefore Q = \frac{1.486}{n} \times \frac{A^{5/3}}{P^{2/3}} \times 5^{1/2}$$

For B.H. \$

$$\frac{(b+o.58y)y}{(b+2.3y)} = \frac{y}{2}$$

$$\frac{2b+1.16y=b+2.3y}{(b=1.14y)}$$

$$A = (1.14y+o.58y)y=1.72y^{2}$$

$$P = 1.14y+2.3y=3.44y$$

$$8000 = \frac{1.486}{0.016} \times \frac{(1.72y^{2})^{5/3}}{(3.44y)^{7/3}} \times (12\times10^{-5})^{1/2}$$

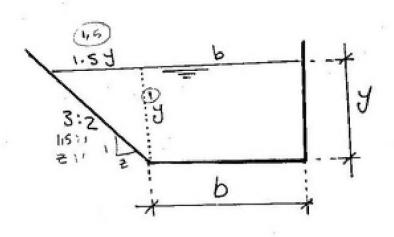
$$7863 = \frac{2.47}{2.28} \frac{y^{19/3}}{2.28}$$

$$y = 28.00 \text{ ft} \text{ #}$$

$$b = 32.00 \text{ ft} \text{ #}$$

## J (6) :

#### Given:



#### 1 - B. J for (B. H.S) Keg. . 2 - bed slope 5

نظماً لذر العظاع لين مسر لعظاعات بلعوفة فيتم ا نبات الشرط الذى يجعل لقطاع (B. H.S) أولاً تم استغرام حدا المسرط في لحل بعد ذلك.

$$P = J + b + \sqrt{(1.5J)^{2} + J^{2}}$$

$$= J + b + \sqrt{3.25J^{2}}$$

$$P = J + b + 1.8J$$

$$P = 2.8J + b \longrightarrow 2$$

$$From 0 b = \frac{A - 0.75J^{2}}{J}$$

$$Subis in 2 \qquad P = 2.8J + \frac{A}{J} - 0.75J$$

$$\therefore P = \frac{A}{J} + 2.05J$$

$$For 8.H.S \frac{dP}{dJ} = 0$$

$$\therefore 0 = -\frac{A}{J^{2}} + 2.05$$

$$\frac{A}{J^{2}} = 2.05$$

$$A = 2.05J^{2}$$

$$b \cdot y + o \cdot 75y^{2} = 2.05y^{2}$$

$$b \cdot y = 1.30 y^{2}$$

$$b = 1.3y$$

$$0 = A \times V$$

$$20 = A \times 0.5 \implies A = 40m^{2}$$

$$b \cdot y + o \cdot 75y^{2} = 40$$

$$1.3y^{2} + o \cdot 75y^{2} = 40$$

$$2.05y^{2} = 40$$

$$y = 4.42 m \#$$

$$b = 5.75 m \#$$

$$b = 5.75 m \#$$

$$c = \frac{A^{5|3}}{P^{2}/3} \cdot 5^{1/2}$$

$$20 = \frac{1}{0.025} \times \frac{(40)^{5/3}}{(18.13)^{2/3}} \times 5^{1/2}$$

$$5^{1/2} = 7.37 \times 10^{-3}$$

$$5 = 5.44 \times 10^{-5}$$

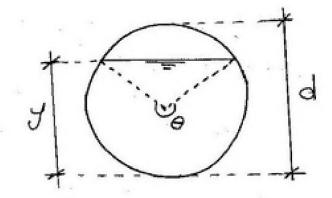
$$5 = 5.44 \text{ Cm}/\text{km} \#$$

Q(7) :

Drive that for

Qmax in circular

Canal 4=0.95d



501.

for amax 
$$\frac{d}{d\theta} \left( \frac{A^3}{P} \right) = 0$$

$$A = \frac{d^2}{8} (\theta_r - \sin \theta) \longrightarrow 1$$

$$\frac{dA}{d\theta} = \frac{d^2}{8} (1 - \cos \theta) \longrightarrow 2$$

$$\rho = \frac{d}{2} \theta_r \longrightarrow 3$$

$$\frac{dP}{d\theta} = \frac{d}{2} \qquad - \Rightarrow 4$$

	8	200	250	300	
	R.H.s				
10.0000	L.H.5				

